

# PLANK-AND-BEAM FRAMING FOR RESIDENTIAL BUILDINGS



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## INTRODUCTION

The plank-and-beam method for framing floors and roofs has been used in heavy timber buildings for many years. The adaptation of this system to residential construction has raised many technical questions from designers and builders concerning the details of application. This publication presents technical data that will be helpful to students, architects, engineers and builders. It contains information pertaining to principles of design, advantages and limitations, construction details, and structural requirements for the plank-and-beam method of framing.

## GENERAL DESCRIPTION

Whereas conventional framing utilizes joists, rafters and studs spaced 12 to 24 inches on center, the plank-and-beam method requires fewer and larger sized pieces spaced farther apart. A simple comparison of the two methods is shown in Figure 1.

In plank-and-beam framing, plank subfloors or roofs, usually of 2-inch nominal thickness, are supported on beams spaced up to 8 feet apart. The ends of the beams are supported on posts or piers. Wall spaces between posts are provided with supplementary framing to the extent required for attachment of exterior and interior finish. This supplementary framing and its covering also serve to provide lateral bracing for the building.

## PRINCIPLES OF DESIGN

The most successful plank-and-beam houses are those which are designed from the beginning for this method of framing. Such procedure permits the correlation of the structural framework with the exterior dimensions of the house, the location of doors and windows and the location of interior partitions. Proper study of these features in the early stages will contribute much to simplified framing.

The most efficient use of 2-inch plank occurs when it is continuous over more than one span. Where standard lengths of lumber are used, such as 12, 14 or 16 feet, beam spacings of 6, 7 or 8 feet are indicated and this has bearing on the overall dimensions of the house. Where end joints in the plank are allowed to occur between supports, tongued-and-grooved or splined random length planks may be used and the beam spacing adjusted to fit the dimensions of the structure. Where square edged planks are used for sub-flooring, recommendations of the National Oak Flooring Manufacturing Association should be followed.

Windows and doors should be located between posts in exterior walls to eliminate the need for headers over

the openings. The wide spacing between posts permits ample opportunity for large glass areas. However, a sufficient amount of solid wall should be present to provide adequate lateral bracing.

Combination of conventional framing with plank-and-beam framing is sometimes used. Where the two adjoin each other on a side-by-side basis, no particular problems are encountered. Where a plank-and-beam floor or roof is supported on a stud wall, a post should be placed under the end of the beam to carry the concentrated load. Where conventional roof framing is used with plank-and-beam construction, a header should be installed to carry the load from the rafters to the posts.

## ADVANTAGES OF SYSTEM

There are many advantages to be gained through the use of the plank-and-beam system of framing. In many houses the roof planks serve as the ceiling, thereby providing added height to living areas with no increase in cost. Where planks are selected for appearance, no further ceiling treatment is needed except the application of a stain, sealer or paint, which may result in substantial cost savings.

Plank-and-beam framing permits substantial savings in labor. The pieces are larger and there are fewer of them than in conventional framing. Cross-bridging of joists is eliminated entirely. Larger and fewer nails are required. All of this adds up to labor savings at the job site.

## LIMITATIONS OF SYSTEM

There are limitations on the use of the plank-and-beam system, but they are readily resolved through careful study in the planning stage. When this is done the parts of the house fit together very quickly and easily.

The plank floors are designed for moderate uniform loads and are not intended to carry heavy concentrated loads. Where such loads occur as those for bearing partitions, bathtubs, refrigerators, etc., additional framing is needed beneath the planks to transmit the loads to the beams.

Insulation is often installed to meet energy requirements. Appearance may be a factor when insulation is installed on the underside of the planking. When insulation is installed over planking, the insulation should be of the rigid type so that it will not deform under load. Rigid type insulation is usually placed over mastic and is limited in roof slope as recommended by the insulation manufacturer.

Location of the electrical distribution system may present a problem because of the lack of concealed spaces in

the ceiling. However, the main supporting beams may be made of several pieces of 2-inch lumber and separated by short blocking, which provides a space to accommodate electrical cable. This is illustrated in Figure 2.

## CONSTRUCTION DETAILS

The plank-and-beam system is essentially a skeleton framework. Planks are destined to support a moderate uniformly distributed load. This is carried to the beams, which in turn transmit their loads to posts, which are supported on the foundation. Where heavy concentrated loads occur in places other than over main beams or posts, supplementary beams are needed to carry such loads. Structural details of the plank-and-beam system of framing are illustrated in Figures 1 through 20.

Foundations for plank-and-beam framing may be continuous walls or piers, supported on adequate footings. With posts spaced up to 8 feet apart in exterior walls, this system is well adapted to pier foundations for houses without basements.

Posts should be of adequate size to carry the load and large enough to provide full bearing for the ends of beams. In general, posts should be at least 4x4 inches, nominal. Where the ends of beams abut over a post, a minimum dimension of 6 inches parallel to the beams is recommended for the post. The posts may be solid or made up of several pieces of 2-inch lumber spiked together.

The size of beams will vary with the span and spacing as indicated in the tables included herein. Beams may be solid, glued laminated pieces, or may be built up of several thinner pieces securely nailed to each other or to spacer blocks between them. When built-up beams are used, a cover plate attached to the underside provides the appearance of a solid piece as illustrated in Figure 3. Fastening of beams to posts is accomplished by framing anchors or angle clips.

Since the 2-inch plank floor or roof frequently serves as the finish ceiling for the room below, appearance as well as structural requirements of the plank should be considered. For the purpose of distributing load, center matched tongued-and-grooved or grooved-for-spline lumber is required. Methods for making the joint to provide various architectural effects are shown in Figure 4. To provide a pleasing appearance, a reasonably good grade of lumber should be selected and it should be sufficiently seasoned to meet the requirements of service conditions so as to avoid large cracks at the joints.

In laying the plank, greater advantage can be taken of the strength and stiffness of the material by making the planks continuous over more than one span. For example, using the same span and uniform load in each case, a plank

which is continuous over two spans is nearly two and one-half times as stiff as a plank which extends over a single span. Planks should be nailed to each support with a minimum of two 16d nails. The finish floor should be laid at right angles to the plank subfloor, using the same procedure followed in conventional construction. Where the underside of the plank is to serve as a ceiling, care is needed to make sure that flooring nails do not penetrate through the plank.

Partitions in the plank-and-beam system usually will be non-bearing. Where bearing partitions occur, they should be placed over beams and the beams enlarged to carry the added load. If this is not possible, supplementary beams must be placed in the floor framing arrangement. Non-bearing partitions, which are parallel to the planks, should have support to carry this load to the beams. This may be accomplished by using two nominal 2x pieces set on edge as the sole plate. Where openings occur in the partition, the two nominal 2x pieces may be placed under the plank floor and supported on the beams by framing anchors. This method is illustrated in Figures 5 and 6. Where the non-bearing partition is at right angles to the planks, no supplementary framing is needed since the partition load will be distributed across a number of planks.

As in conventional framing, lateral bracing is required in the exterior walls to provide resistance against wind and seismic forces. In plank-and-beam framing, this is accomplished by installing solid panels at appropriate intervals wherein the supplementary wall framing and the posts are all tied together by diagonal bracing or suitable sheathing.

## STRUCTURAL REQUIREMENT

Good design requires that all members be properly fastened together in order that the house will act as a unit in resisting external forces. With fewer pieces than in conventional framing, particular care must be given to connections where beams abut each other and where beams join the posts. Where gable roofs are used, provision must be made to absorb the horizontal thrust produced by sloping roof beams. Methods for doing this are shown in the illustrations included herein.

In most cases, structural design of the plank-and-beam house will be controlled by the local building code to the extent of specifying design loading requirements. A live load of 40 pounds per square foot is commonly specified for floors. For roofs, some codes specify 20 pounds per square foot and others 30 pounds per square foot.

To provide adequate safety, all codes require that framing members be so proportioned that the allowable fiber



stress in bending is not exceeded when the member is subjected to full live and dead loads. However, from the standpoint of appearance, most designers and builders prefer to place some limit on the allowable deflection.

Tables indicating allowable loads for planks and beams will be found herein. Allowable loads for posts will be found in *Wood Structural Design Data*, a publication of the American Forest and Paper Association.

## DESIGN DATA FOR PLANKS

Design data for plank floors and roofs are included in Table 1. Computations for bending are based on the live load indicated, plus 10 or 20 pounds per square foot of

dead load. Computations for deflection are based on the live load only. Tabulated modulus of elasticity and bending stresses do not account for partial live loading on adjacent and alternate spans. The table shows four general arrangements of planks as follows:

- Type A — Extending over a single span.
- Type B — Continuous over two equal spans.
- Type C — Continuous over three equal spans.
- Type D — A combination of Types A and B.

On the basis of a section of planking 12 inches wide the following formulas were used in making the computations:

$$\text{For Type A: } M = \frac{wL^2}{8} \text{ and } \Delta = \frac{5wL^4(12)^3}{384EI}$$

$$\text{For Type B: } M = \frac{wL^2}{8} \text{ and } \Delta = \frac{wL^4(12)^3}{185EI}$$

$$\text{For Type C: } M = \frac{wL^2}{10} \text{ and } \Delta = \frac{4wL^4(12)^3}{581EI}$$

$$\text{For Type D: } M = \frac{wL^2}{8} \text{ and } \Delta = \frac{1}{2} \left( \frac{5wL^4(12)^3}{384EI} + \frac{wL^4(12)^3}{185EI} \right)$$

To use Table 1, first determine the plank arrangement (Types A, B, C or D), the span, the live load to be supported and the deflection limitation. Then select from the table the corresponding required values for fiber stress in bending ( $f_b$ ) and modulus of elasticity (E). The plank to be used should be of a grade and species that meets these minimum values. The maximum span for a specific grade and species of plank may be determined by reversing these steps.

For those who prefer to use random length planks (instead of arrangements Type A, B, C or D), similar technical information is included in *Heavy Timber Construction Details*, a publication of the American Forest and Paper Association

## DESIGN DATA FOR BEAMS

Design data for beams are included in Tables 2 through 10. Computations for bending are based on the live load indicated plus 10 or 20 pounds per square foot of dead load. Computations for deflection are based on the live load only. All beams in the table were designed to extend over a single span and the following formulas were used:

$$\text{For Type A: } M = \frac{wL^2}{8} \text{ and } \Delta = \frac{5wL^4(12)^3}{384EI}$$

To use the tables first determine the span, the live load to be supported, and the deflection limitation. Then select from the tables the proper size of beam with the corresponding required values for fiber stress in bending ( $f_b$ ) and modulus of elasticity (E). The beam used should be of a grade and species that meets these minimum value. The maximum span for a beam of specific size, grade and species can be determined by reversing these steps.

## NOTATIONS

In the preceding formulas and in the tables the symbols have the following meanings:

w = load (plf)

L = span (ft.)

M = induced bending moment (lbs.-ft.)

$f_b$  = bending stress (psi)

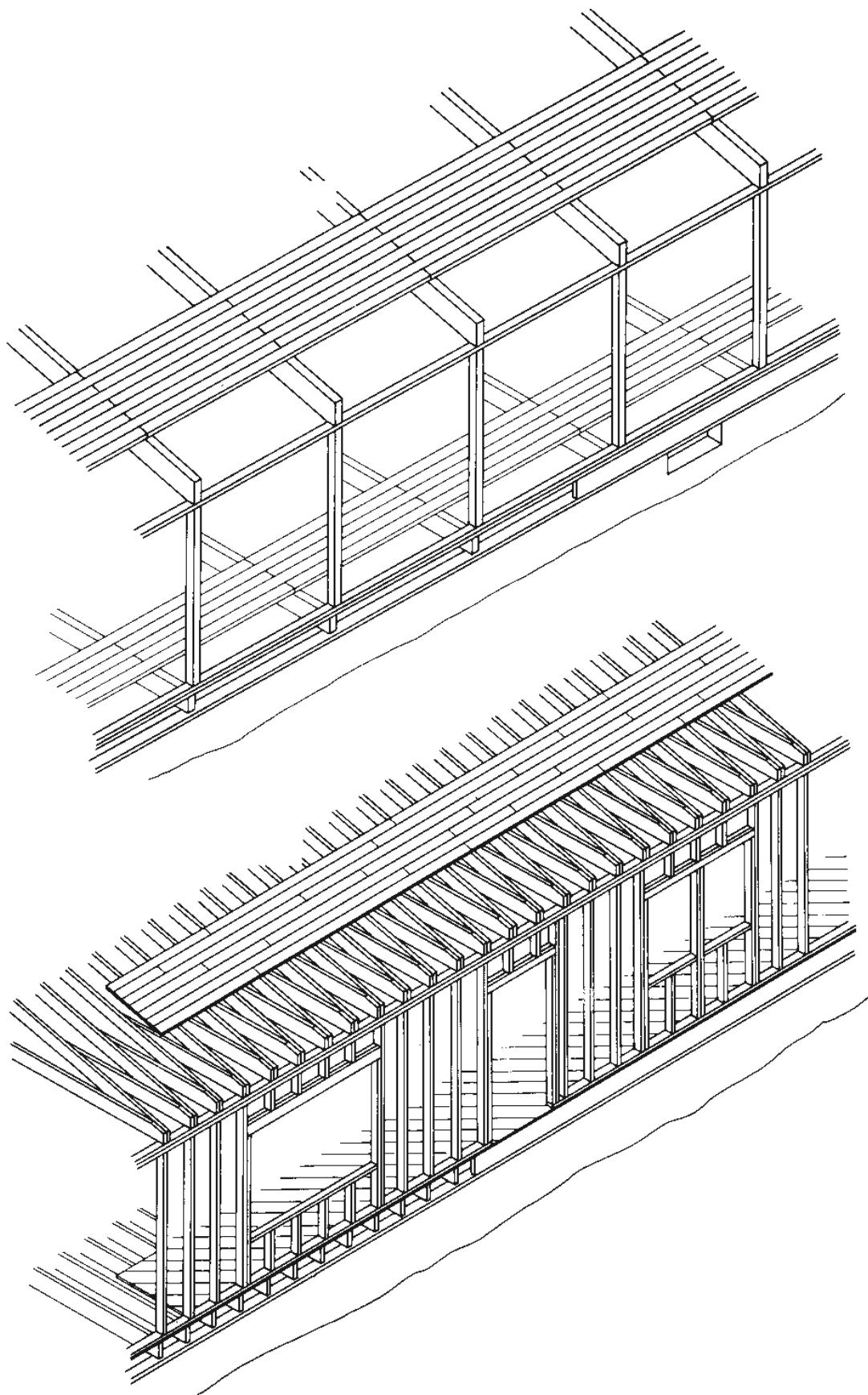
E = modulus of elasticity (lbs.-in.)

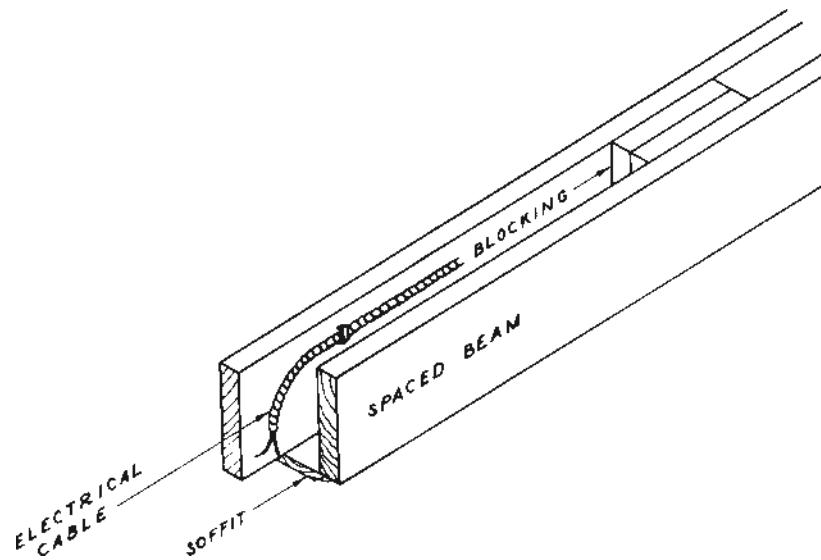
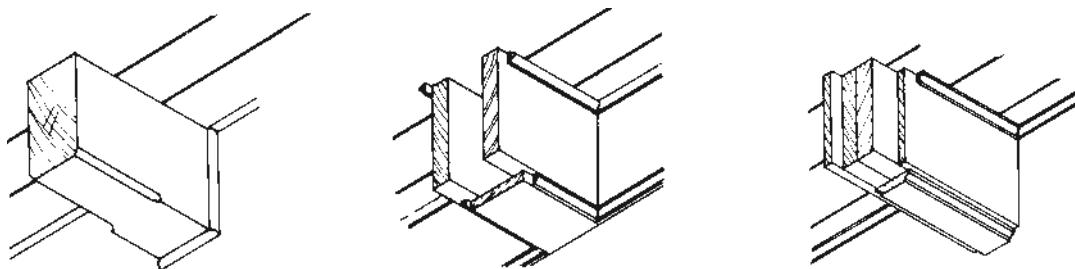
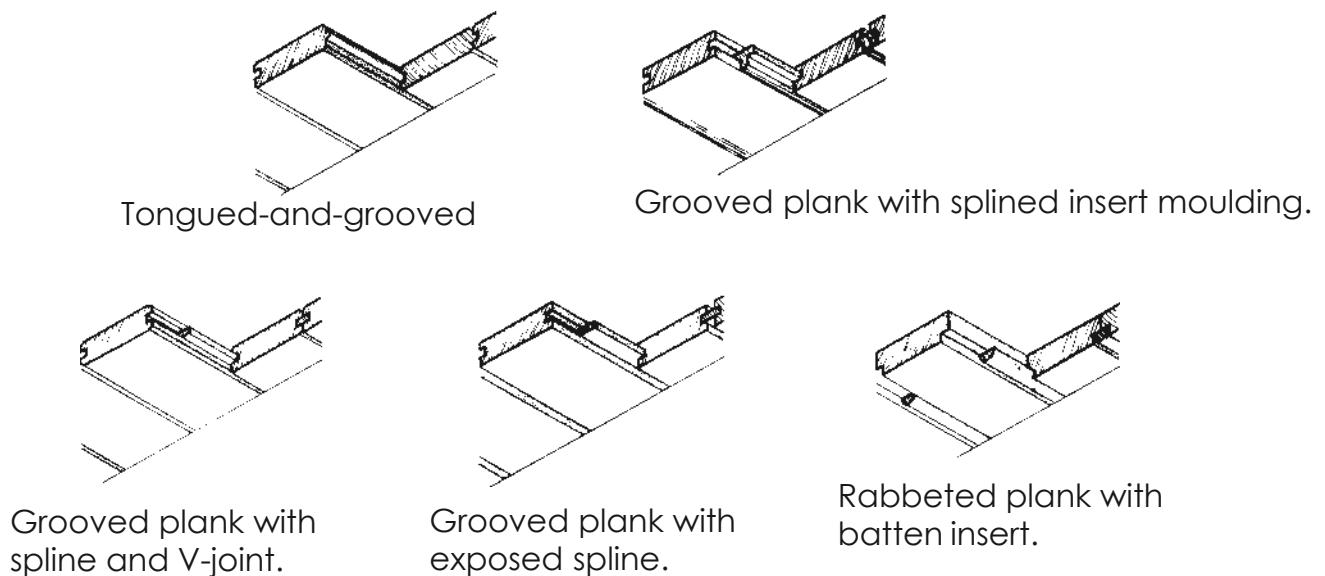
I = moment of inertia (in.<sup>4</sup>)

$\Delta$  = deflection (in.)

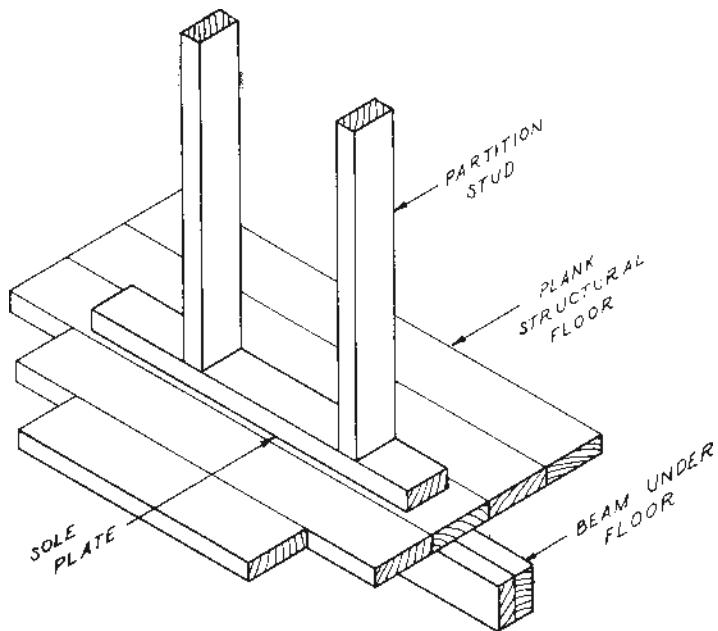
## LUMBER SIZES

Tabular data provided herein are based on net dimensions (S4S) as listed in American Softwood Lumber Standard, VPS 20-99.

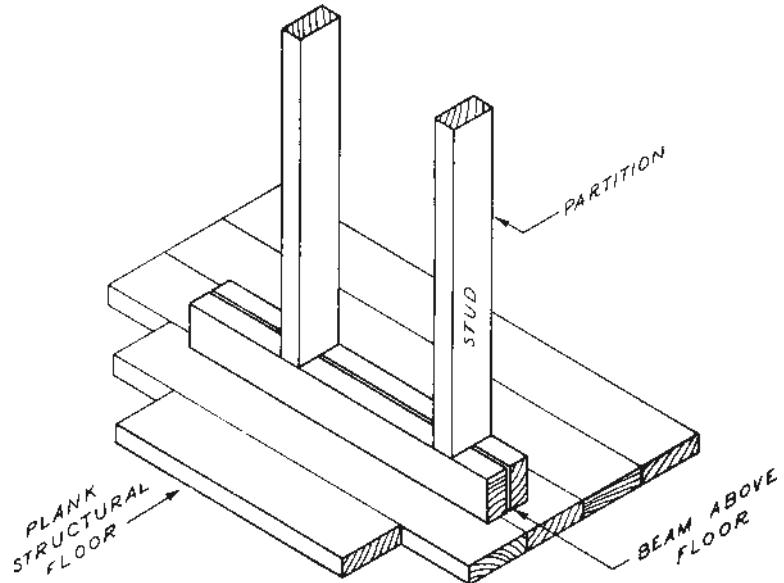
**Figure 1 Comparison of Plank-and-beam System With Conventional Framing**

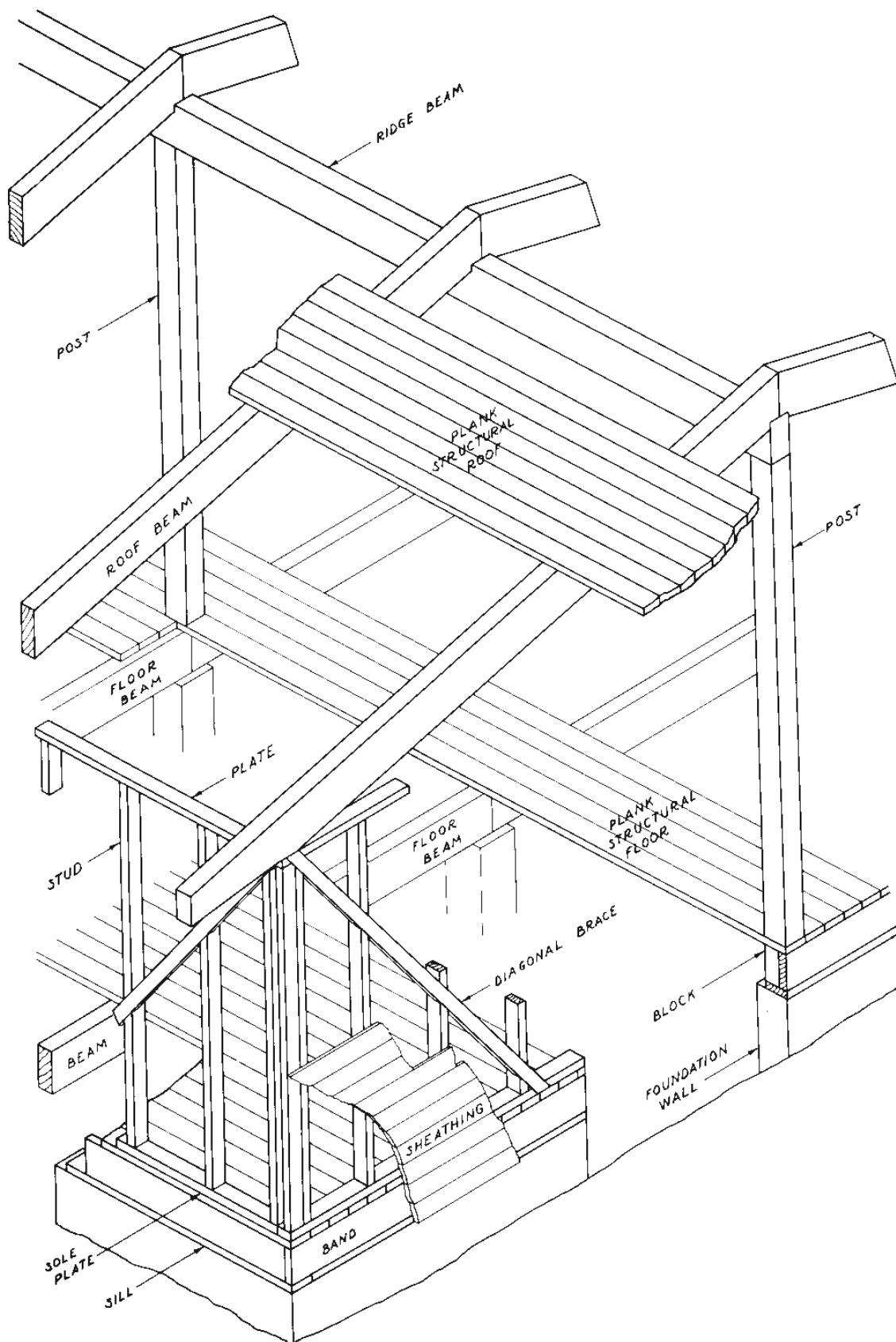
**Figure 2 Use of Spaced Beam to Accommodate Electrical Cable****Figure 3 Methods of Finishing Undersides of Beams****Figure 4 Methods of Treating Joints in Exposed Plank Ceilings**

**Figure 5 Support for Non-bearing Partition Parallel to Plank With Beam Under Floor**

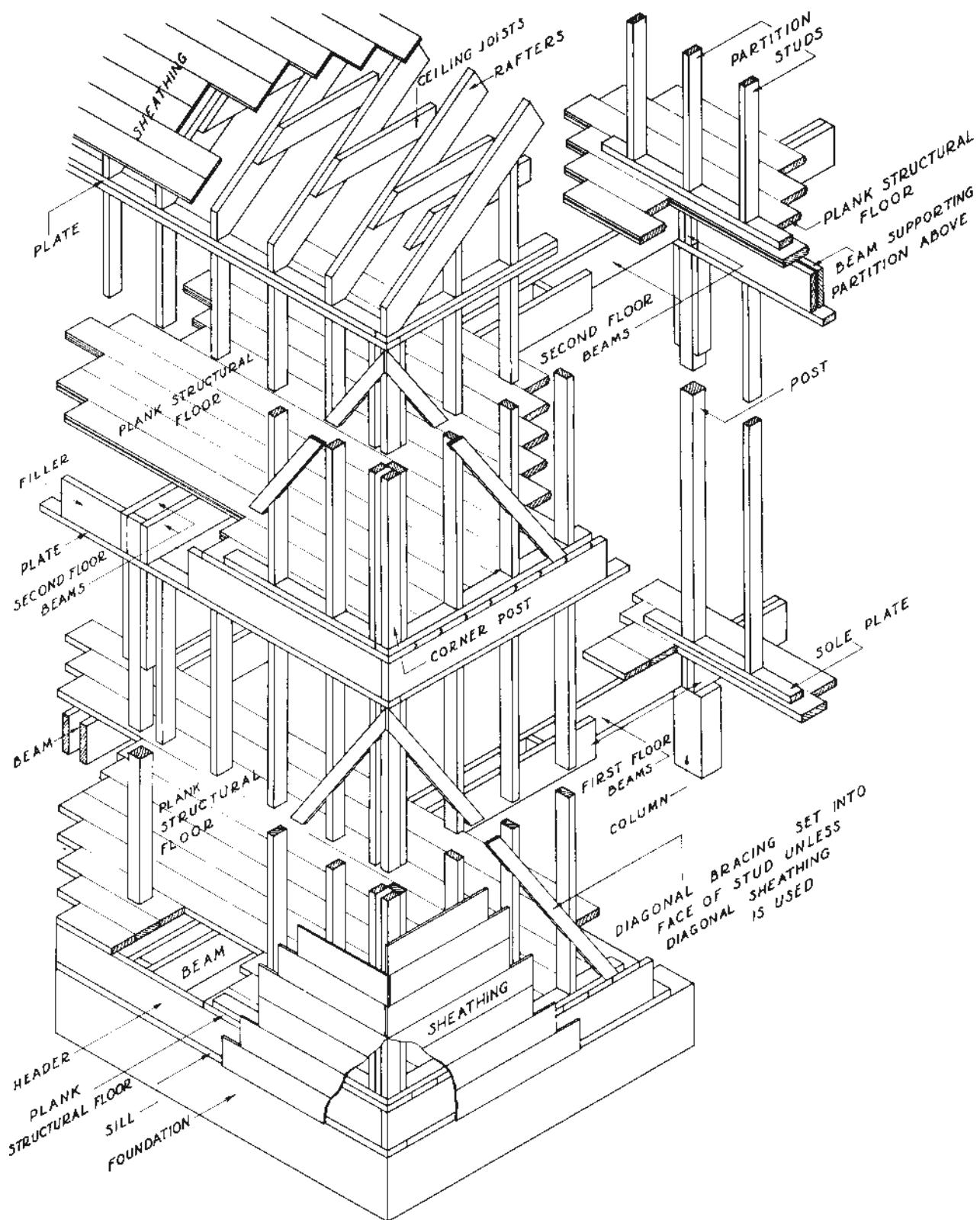


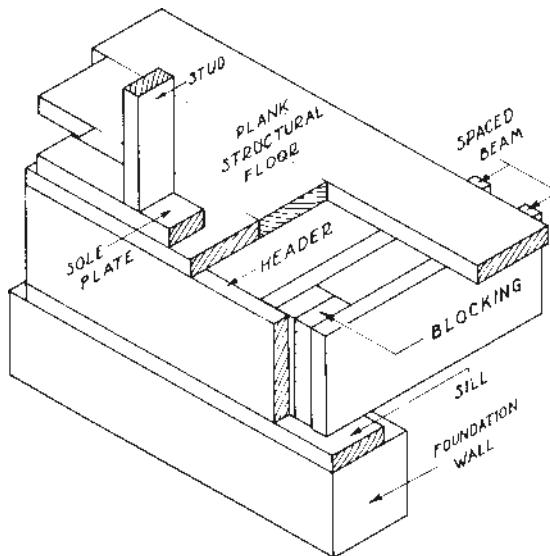
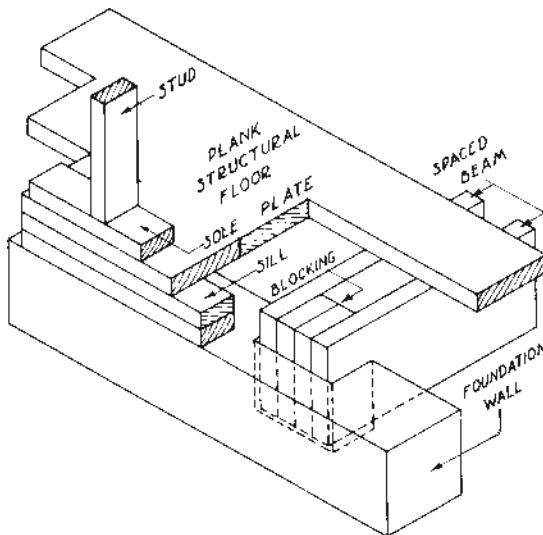
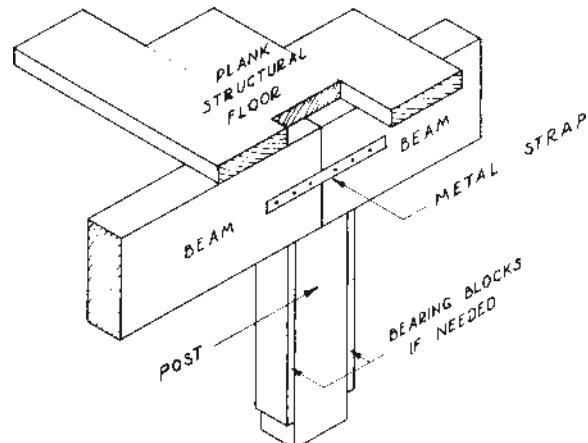
**Figure 6 Support for Non-bearing Partition Parallel to Plank With Beam Above Floor**

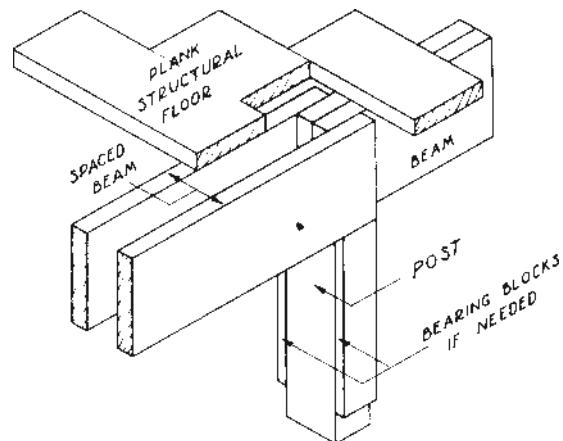
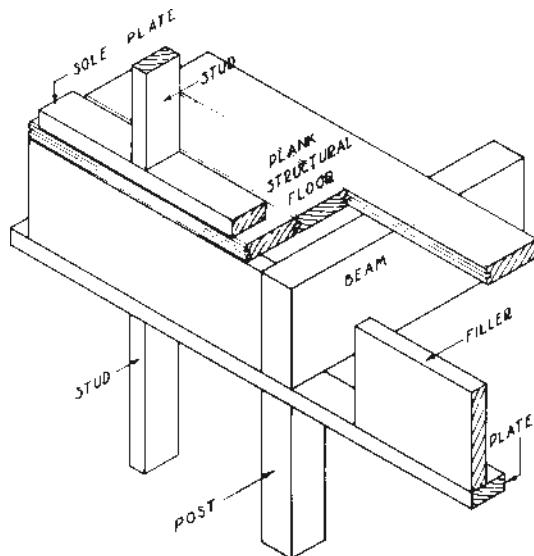
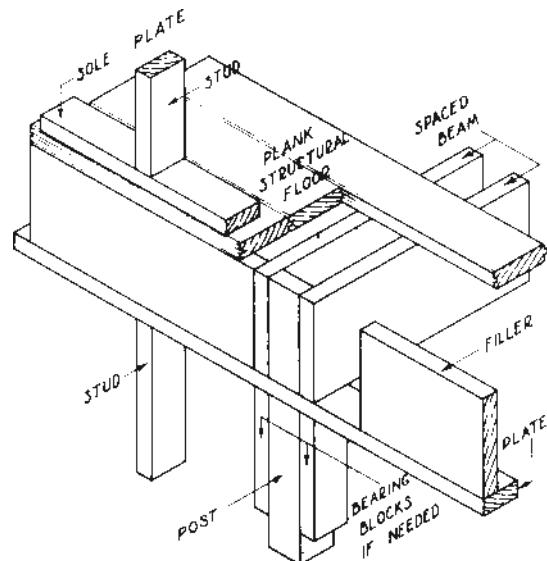


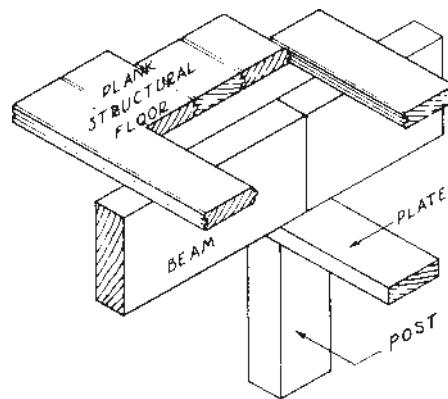
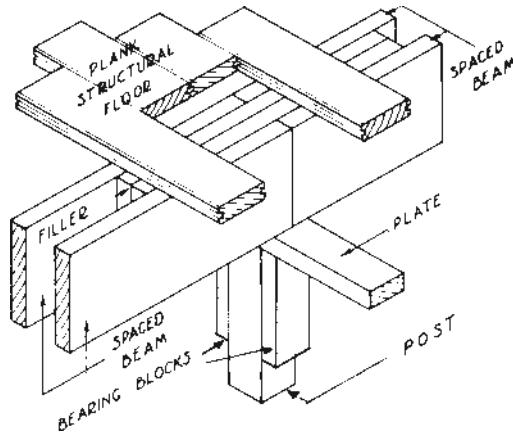
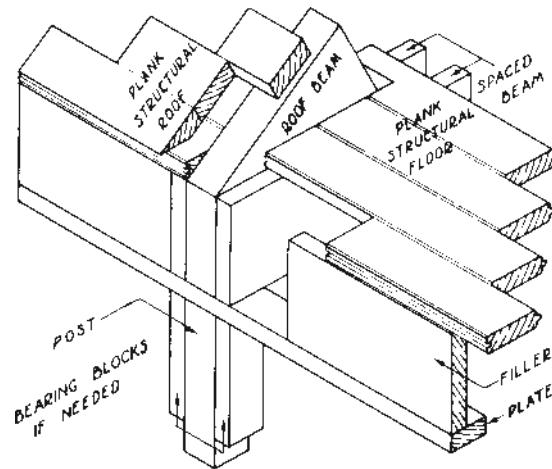
**Figure 7 Plank-and-beam Framing for One-story House**

**Figure 8 Plank-and-beam Framing Combined With Conventional Framing in Two-story House**

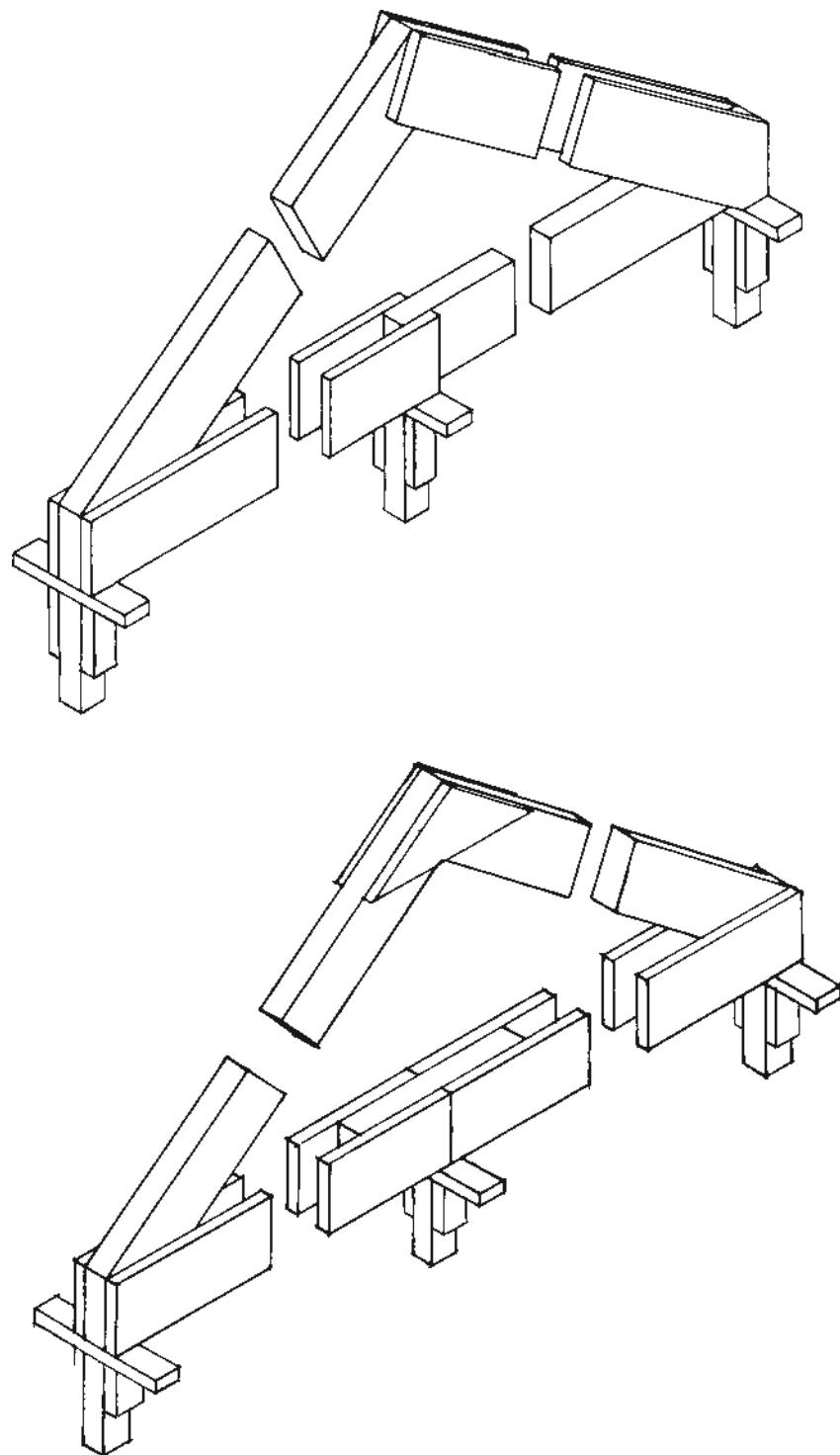


**Figure 9 First Floor Framing at Exterior Wall With Beam Bearing on Sill****Figure 10 First Floor Framing at Exterior Wall With Beam Set in Foundation Wall****Figure 11 Solid Beam Bearing Over Basement Post**

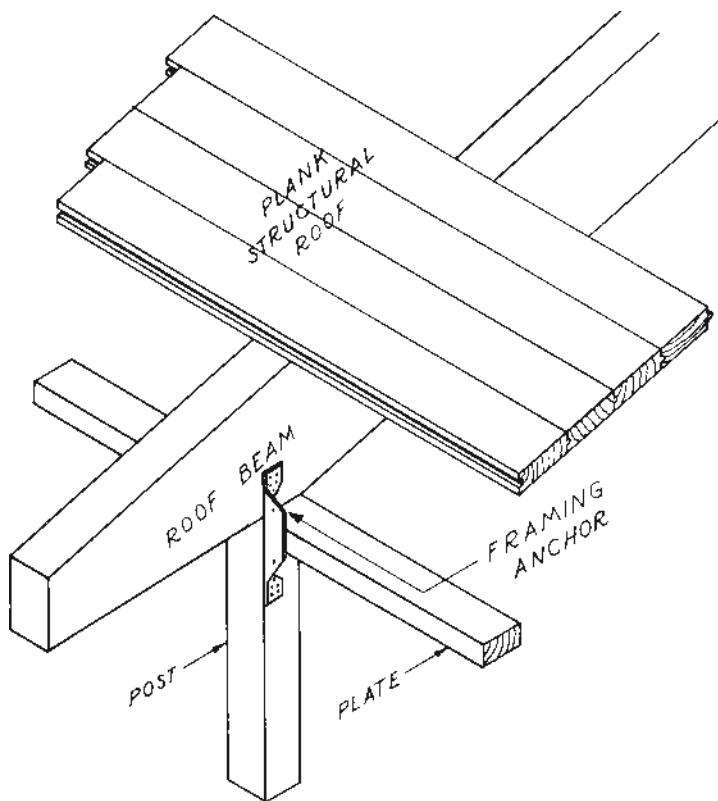
**Figure 12 Spaced Beam Bearing Over Basement Post****Figure 13 Beam Bearing at Second Floor Exterior Wall****Figure 14 Spaced Beam Bearing at Second Floor Exterior Wall**

**Figure 15 Solid Beam at Second Floor Over Interior Post****Figure 16 Spaced Beam Bearing at Second Floor Over Interior Post****Figure 17 Roof Beam and Spaced Floor Beam Bearing at Exterior Wall**

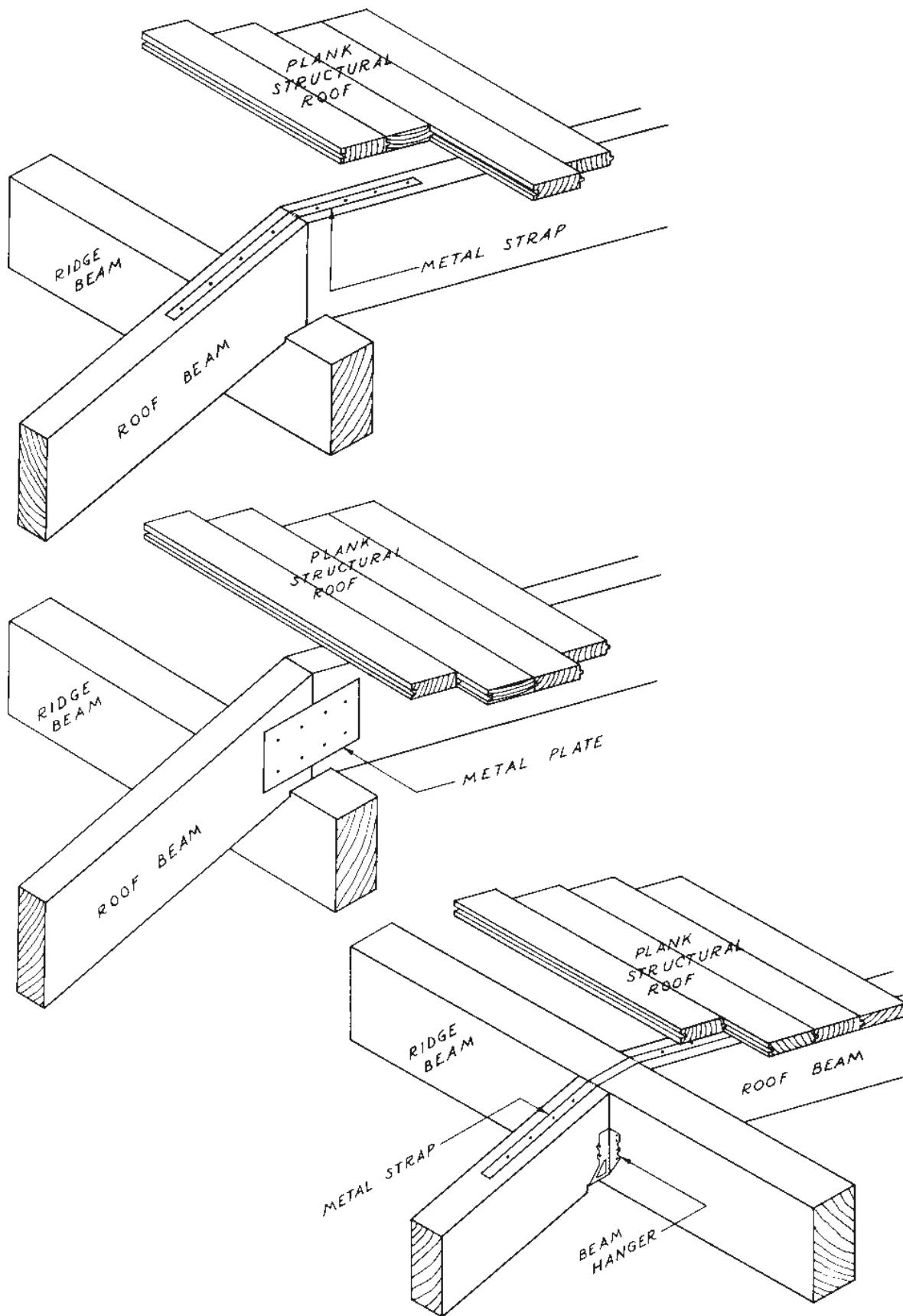
**Figure 18 Arrangement of Roof and Floor Beams to Absorb Horizontal Thrust**



**Figure 19 Bearing of Solid Roof Beam on Exterior Wall Post**



**Figure 20 Methods of Supporting Roof Beam on Ridge Beam to Absorb Horizontal Thrust**





**Table 1 Nominal Two-Inch Plank.**

Required values for fiber stress in bending ( $f_b$ ) and modulus of elasticity (E) to support safely a live load of 20, 30, or 40 pounds per square foot within a deflection limitation of L/180, L/240 or L/360. For instructions on use of table see Design Data for Planks on page 3.

Plank Span (ft.)	Live Load (psf)	Deflection Limitation	Type A		Type B		Type C		Type D					
			$f_b$ (psi)		E (psi)	$f_b$ (psi)		E (psi)	$f_b$ (psi)		E (psi)	$f_b$ (psi)		
			10 psf DL	20 psf DL		10 psf DL	20 psf DL		10 psf DL	20 psf DL		10 psf DL	20 psf DL	
6	20	L/180	360	480	430,000	360	480	180,000	288	384	230,000	360	480	310,000
		L/240	360	480	580,000	360	480	240,000	288	384	300,000	360	480	410,000
		L/360	360	480	860,000	360	480	360,000	288	384	460,000	360	480	610,000
	30	L/180	480	600	650,000	480	600	270,000	384	480	340,000	480	600	460,000
		L/240	480	600	860,000	480	600	360,000	384	480	460,000	480	600	610,000
		L/360	480	600	1,300,000	480	600	540,000	384	480	690,000	480	600	920,000
	40	L/180	600	720	860,000	600	720	360,000	480	576	460,000	600	720	610,000
		L/240	600	720	1,150,000	600	720	480,000	480	576	610,000	600	720	820,000
		L/360	600	720	1,730,000	600	720	720,000	480	576	910,000	600	720	1,220,000
7	20	L/180	490	653	690,000	490	653	280,000	392	523	360,000	490	653	490,000
		L/240	490	653	910,000	490	653	380,000	392	523	480,000	490	653	650,000
		L/360	490	653	1,370,000	490	653	570,000	392	523	730,000	490	653	970,000
	30	L/180	653	817	1,030,000	653	817	430,000	523	653	540,000	653	817	730,000
		L/240	653	817	1,370,000	653	817	570,000	523	653	730,000	653	817	970,000
		L/360	653	817	2,060,000	653	817	850,000	523	653	1,090,000	653	817	1,460,000
	40	L/180	817	980	1,370,000	817	980	570,000	653	784	730,000	817	980	970,000
		L/240	817	980	1,830,000	817	980	760,000	653	784	970,000	817	980	1,290,000
		L/360	817	980	2,740,000	817	980	1,140,000	653	784	1,450,000	817	980	1,940,000
8	20	L/180	640	853	1,020,000	640	853	430,000	512	683	540,000	640	853	720,000
		L/240	640	853	1,370,000	640	853	570,000	512	683	720,000	640	853	970,000
		L/360	640	853	2,050,000	640	853	850,000	512	683	1,080,000	640	853	1,450,000
	30	L/180	853	1,067	1,540,000	853	1,067	640,000	683	853	810,000	853	1,067	1,090,000
		L/240	853	1,067	2,050,000	853	1,067	850,000	683	853	1,080,000	853	1,067	1,450,000
		L/360	853	1,067	3,070,000	853	1,067	1,280,000	683	853	1,620,000	853	1,067	2,170,000
	40	L/180	1,067	1,280	2,050,000	1,067	1,280	850,000	853	1,024	1,080,000	1,067	1,280	1,450,000
		L/240	1,067	1,280	2,730,000	1,067	1,280	1,130,000	853	1,024	1,440,000	1,067	1,280	1,930,000
		L/360	1,067	1,280	4,100,000	1,067	1,280	1,700,000	853	1,024	2,170,000	1,067	1,280	2,900,000

























**Table 8 Floor and Roof Beams**

Required values for fiber stress in bending ( $f_b$ ) and modulus of elasticity (E) for the sizes shown to support safely a live load of 40 pounds per square foot within a deflection limitation of L/180. For instructions on use of table see Design Data For Beams on page 4.

Beam Span (ft.)	Nominal Size	Plank Span = 6'-0"			Plank Span = 7'-0"			Plank Span = 8'-0"		
		$f_b$ (psi)		E (psi)	$f_b$ (psi)		E (psi)	$f_b$ (psi)		E (psi)
		10 psf DL	20 psf DL		10 psf DL	20 psf DL		10 psf DL	20 psf DL	
10	2-3x6	1,785	2,140	1,170,000	2,085	2,500	1,360,000	2,380	2,855	1,560,000
	1-3x8	2,055	2,465	1,020,000	2,395	2,875	1,190,000	2,740	3,290	1,360,000
	2-2x8	1,710	2,055	850,000	2,000	2,395	990,000	2,285	2,740	1,130,000
	1-4x8	1,470	1,760	730,000	1,710	2,055	850,000	1,955	2,350	970,000
	1-6x8	875	1,045	420,000	1,020	1,220	490,000	1,165	1,395	560,000
	2-2x10	1,050	1,260	410,000	1,225	1,475	480,000	1,400	1,685	550,000
11	1-3x10	1,260	1,515	490,000	1,475	1,765	570,000	1,685	2,020	650,000
	2-2x8	2,070	2,485	1,130,000	2,415	2,900	1,320,000	2,760	3,315	1,510,000
	1-4x8	1,775	2,130	970,000	2,070	2,485	1,130,000	2,370	2,840	1,290,000
	1-6x8	1,055	1,265	560,000	1,230	1,480	650,000	1,410	1,690	740,000
	2-2x10	1,275	1,525	540,000	1,485	1,780	640,000	1,695	2,035	730,000
	1-3x10	1,525	1,835	650,000	1,780	2,140	760,000	2,035	2,445	870,000
12	1-4x10	1,090	1,310	470,000	1,275	1,525	540,000	1,455	1,745	620,000
	3-2x10	850	1,020	360,000	990	1,190	420,000	1,130	1,360	480,000
	1-4x8	2,115	2,535	1,260,000	2,465	2,960	1,470,000	2,820	3,380	1,680,000
	1-6x8	1,255	1,510	720,000	1,465	1,760	840,000	1,675	2,010	970,000
	3-2x8	1,645	1,975	980,000	1,920	2,300	1,140,000	2,190	2,630	1,310,000
	2-2x10	1,515	1,820	710,000	1,765	2,120	830,000	2,020	2,425	940,000
13	1-3x10	1,820	2,180	850,000	2,120	2,545	990,000	2,425	2,910	1,130,000
	1-4x10	1,300	1,560	610,000	1,515	1,820	710,000	1,730	2,075	810,000
	3-2x10	1,010	1,210	470,000	1,180	1,415	550,000	1,345	1,615	630,000
	2-3x10	910	1,090	420,000	1,060	1,270	500,000	1,210	1,455	570,000
	1-6x8	1,475	1,770	920,000	1,720	2,065	1,070,000	1,965	2,360	1,230,000
	2-3x8	1,735	2,085	1,120,000	2,025	2,430	1,310,000	2,315	2,780	1,490,000
14	2-4x8	1,240	1,490	800,000	1,445	1,735	930,000	1,655	1,985	1,070,000
	2-2x10	1,780	2,135	900,000	2,075	2,490	1,050,000	2,370	2,845	1,200,000
	3-2x10	1,185	1,420	600,000	1,385	1,660	700,000	1,580	1,895	800,000
	1-3x10	2,135	2,560	1,080,000	2,490	2,985	1,260,000	2,845	3,415	1,440,000
	2-3x10	1,065	1,280	540,000	1,245	1,495	630,000	1,420	1,705	720,000
	1-4x10	1,525	1,830	770,000	1,780	2,135	900,000	2,030	2,440	1,030,000
15	2-4x10	760	915	390,000	890	1,065	450,000	1,015	1,220	510,000
	3-2x10	1,440	1,725	1,000,000	1,680	2,015	1,170,000	1,920	2,300	1,330,000
	2-3x10	1,375	1,650	750,000	1,605	1,925	870,000	1,835	2,200	1,000,000
	2-3x10	1,235	1,485	670,000	1,445	1,730	790,000	1,650	1,980	900,000
	1-4x10	1,765	2,120	960,000	2,060	2,475	1,120,000	2,355	2,825	1,280,000
	2-4x10	885	1,060	480,000	1,030	1,235	560,000	1,180	1,415	640,000
16	3-3x10	825	990	450,000	960	1,155	520,000	1,100	1,320	600,000
	1-6x10	1,065	1,280	570,000	1,245	1,495	660,000	1,420	1,705	750,000
	1-8x10	780	940	410,000	910	1,095	480,000	1,040	1,250	550,000
	4-2x10	1,030	1,235	560,000	1,205	1,445	660,000	1,375	1,650	750,000
	2-2x12	1,395	1,675	620,000	1,625	1,950	730,000	1,860	2,230	830,000
	3-2x10	1,580	1,895	920,000	1,840	2,210	1,070,000	2,105	2,525	1,230,000











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